GENERAL STRUCTURAL NOTES: G1. ALL STRUCTURAL WORK SHALL COMPLY WITH ALL APPLICABLE FEDERAL, NEW YORK STATE BUILDING CODE 2020 AND LOCAL CODES LAWS & REGULATIONS. RISK AND SHALL BE REMOVED AND REINSTALLED TO THE SPECIFICATIONS OF THE ARCHITECT AT NO ADDITIONAL COST TO THE OWNER. G10. MEANS AND METHODS OF CONSTRUCTION AS WELL AS COMPLIANCE WITH OSHA AND OTHER SAFETY LAWS AND REGULATIONS IS EXCLUSIVE G2. CONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING AND COORDINATING ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. IN CASE OF CONFLICT, THE ARCHITECT SHALL BE NOTIFIED AND SHALL RESOLVE THE CONFLICT. G3. IN ANY CASE OF CONFLICT BETWEEN THE DRAWINGS AND THE PROJECT SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. G4. THE CONTRACTOR SHALL MAKE NO DEVIATION FROM DESIGN DRAWINGS WITHOUT PRIOR REVIEW BY THE ARCHITECT. G5. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE FOUNDATIONS AND BACKFILL: ALL WORK SHALL COMPLY WITH APPLICABLE CODES AND LOCAL LAWS AND REGULATIONS. GENERAL CONTRACTOR SHALL COORDINATE LOCATIONS OF OPENINGS, PITS, BOXES, SUMPS, TRENCHES, SLEEVES, DEPRESSIONS, GROOVES, AND CHAMFERS, WITH MECHANICAL, ELECTRICAL AND PLUMBING TRADES. G8. THE STRUCTURAL DESIGN IS BASED ON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS. NO PROVISIONS HAVE BEEN MADE FOR CONDITIONS OCCURRING DURING CONSTRUCTION. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO MAKE PROPER AND ADEQUATE PROVISIONS FOR STABILITY OF, AND ALL STRESSES TO, THE STRUCTURE DUE TO ANY CAUSE DURING CONSTRUCTION. 15.75" 15,75" 1. The PC Concrete Arches, the Columns, Cap Plate and Base Plates shall be stamped by a NYS Lic Engineer. 2. The two PC Conc $\frac{1}{4}$ Arches shall be 6" CMU—Polished Light Tan: 1 fastened to each other in the Fields with 6" CMU-Polished Dark Tan: 2 10"x5"x½" Galv Stl Plates. Additionally, Weld 6" CMU-Split face Dark Tan: 3 a Galv Stl Plt connected all PC Conc PC Conc Arch/Column-white: 4 Arches together. PC Conc Arch Section $_{1}4$ "x10 $_{2}$ " $_{2}$ " Galv Stl Plt, Weld (Cont $_{4}$ " Fillet Weld) to imbedded Stl Plt on top of PC Conc Arches. 4" CMU-Polished / Light Tan, Typ 4'-74" 4" Brick-Tan, Typ- $18" \times 10" \times \frac{1}{2}$ " Galv Stl Plt, Weld (Cont $\frac{1}{4}$ " Fillet Weld) to Galv Stl Plt on top of PC 6"x6"x½" Galv StI imbedded Plate with 2 Conc Arches. 4"x½" Headed Studs Set in the PC ¼ 4" Brick-Red, Typ TO FL Deck TO BRG-1st FL FFL-Conc Slab Front Elevation PC Conc Arch Front Elevation 2 PC Conc Arch Section $\frac{1}{4}$ " = 1'-0" STRUCTURAL DESIGN CRITERIA: CONCRETE: DC1. CODES: 2015 NEW YORK STATE BUILDING CODE C1. UNLESS OTHERWISE NOTED ALL CONCRETE SHALL BE NORMAL WEIGHT, 3/4" STONE CONCRETE WITH 4000 DC2. DESIGN LOADS: PSI 28 DAY COMPRESSIVE STRENGTH. ALL CONCRETE EXPOSED TO WEATHER AND CONCRETE FOR FOUNDATIONS SHALL BE AIR ENTRAINED 5% TO 7%. #6 AND LARGER: LIVE LOADS: C2. ALL REINFORCING SHALL BE ASTM A-615 GRADE 60. WELDED WIRE FABRIC SHALL BE ASTM A-185. ROOF/CANOPY 20 PSF

G9. CONTRACTOR SHALL NOT SCALE DRAWINGS. CONTRACTOR SHALL REQUEST ALL DIMENSIONS OR INFORMATION REQUIRED TO PERFORM THE WORK FROM THE ARCHITECT. WORK COMPLETED BY THE CONTRACTOR WITHOUT DIMENSIONS OR INFORMATION SHALL BE DONE AT THE CONTRACTOR SOWN

RESPONSIBILITY OF THE CONTRACTOR, HIS SUBCONTRACTOR(S), SUPPLIERS, CONSULTANTS AND SERVANTS.

F1. ALL FOUNDATIONS SHALL BEAR ON UNDISTURBED MATERIAL. THE ARCHITECT SHALL BE NOTIFIED A MINIMUM OF 72 HOURS IN ADVANCE OF FOOTING CONCRETE PLACEMENT.
F2. NO FOOTINGS SHALL BE PLACED IN WATER, NOR UPON FROZEN GROUND.
F3. MATERIAL ADJACENT TO AND BELOW FOOTINGS SHALL BE KEPT FROM FREEZING AT ALL TIMES. IF ANY MATERIAL IS FOUND TO BE FROZEN IT SHALL BE REMOVED AND REPLACED WITH CONCRETE. IF ANY FROZEN MATERIAL IS FOUND BELOW THE SLAB-ON-GRADE, IT SHALL BE REMOVED AND REPLACED WITH

F4. HAND EXCAVATE THE FINAL 6 INCHES OF MATERIAL TO THE BEARING LEVEL AT ALL FOOTINGS.
 F5. ALL FOOTINGS AND PIERS SHALL BE CENTERED UNDER PROPOSED COLUMNS/POSTS UNLESS OTHERWISE NOTED.

ALL EXTERIOR FOUNDATIONS SHALL BE SET AT OR BELOW THE FROST DEPTH (MIN 48" BELOW GARDE).

F7. VERTICAL MISALIGNMENT OF ANCHOR BOLTS SHALL BE HELD TO 1:40 OR LESS.

STRUCTURAL STEEL:

S1. STRUCTURAL STEEL DESIGN, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL BUILDINGS.

S2. STRUCTURAL STEEL SHALL BE NEW STRUCTURAL CARBON STEEL CONFORMING TO:

W-SHAPE: ASTM A992 GRADE 50 STRUCTURAL TUBING: ASTM A500 GRADE B **ANGLES** ASTM A36 **PLATES** ASTM A529 GRADE 50

S3. ALL BEAM-TO-BEAM, BEAM-TO-GIRDER, AND BEAM OR GIRDER-TO-COLUMN

CONNECTIONS SHALL BE PER AISC SPECIFICATIONS.

S4. WELDING SHALL CONFORM TO AWS D1.1, SHALL BE DONE BY CERTIFIED WELDERS AND SHALL BE UNDERTAKEN BY A FABRICATOR QUALIFIED BY THE AWS.

S5. UNLESS OTHERWISE NOTED ALL BOLTED CONNECTIONS SHALL BE SLIP-CRITICAL WITH

SERVICEABILITY AS THE LIMIT-STATE. CLASS A FAYING SURFACES SHALL BE USED AT ALL CONNECTION INTERFACES. ALL BOLTS SHALL BE 3/4 INCH DIAMETER HIGH-STRENGTH TENSION CONTROL BOLTS CONFORMING TO ASTM F1852 AND GALVANIZED, UNLESS OTHERWISE NOTED.

S6. ALL ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GRADE 55. ALL ANCHOR BOLTS SHALL BE 1 3/4" DIAMETER AND GALVANIZED, UNLESS OTHERWISE NOTED.

S7. ALL WELDS SHALL BE 1/4" FILLET WELDS UNLESS OTHERWISE NOTED OR THE AISC MINIMUM WELD SIZE IS GREATER. ALL WELDING ELECTRODES SHALL BE GRADE E-70. S8. ALL STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED

S9. ALL CUTS, HOLES, COPINGS, ETC. REQUIRED IN THE STEEL SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTS, COPINGS, OR BURNING OF HOLES, ETC. IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED.

S10. TEMPORARY BRACING OF STEEL MEMBERS DURING CONSTRUCTION IS REQUIRED AND

IS THE RESPONSIBILITY OF THE CONTRACTOR.
S11. VERIFICATION OF ADEQUACY OF ANCHOR BOLTS AND FOUNDATIONS TO RESIST

ECTION INDUCED FORCES IS SOLELY THE RESPONSIBILITY OF THE STEEL ERECTOR. S12. ALL SHOP AND FIELD WELDING IS SUBJECT TO INSPECTION. COMPLETE PENETRATION WELDS IN MOMENT-RESISTING CONNECTIONS SHALL BE TESTED BY ULTRASOUND OR RADIOGRAPHY AS PART OF THE PROGRAM OF STRUCTURAL TESTS AND INSPECTIONS.

WOOD NOTES

1.) WOOD CONSTRUCTION SHALL CONFORM TO THE AMERICAN FOREST and PAPER ASSOCIATION'S (AF&PA) NATIONAL DESIGN SPECIFICATIONS, 2001 EDITION. LUMBER SHALL BE #2 HEM-FIR OR BETTER WITH Fb=850 psi, Fv=150 psi AND E=1,300,000 psi.

2.) WOOD IN CONTACT WITH MASONRY, CONCRETE OR EARTH, OR WITHIN 1'-0" OF GRADE OR EXPOSED TO THE EXTERIOR SHALL BE PRESSURE PRESERVATIVE TREATED.

3.) MICRO-LAM (LVL) LUMBER AND TRUS-JOISTS SHALL BE AS MANUFACTURED BY "TRUS-JOIST, LVL by Weyerhaeuser". BEAMS SHALL BE PROPERLY FASTENED TOGETHER WITH A MINIMUM OF (2) -ROWS OF 16d NAILS PER FOOT. INSTALL PER MANUFACTURERS

4.) FRAMING ANCHORS AND MISCELLANEOUS METAL CONNECTING DEVICES FOR WOOD FRAMING SHALL BE GALVANIZED STEEL OF AT LEAST 16 GAGE THICKNESS INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. USE NAILS SUPPLIED BY OR RECOMMENDED BY THE MANUFACTURER.

All wood in contact with Concrete , Masonry, openly exposed to weather shall be Pressure Treated. Provide ¹/₈" Thick Closed Cell compressible foam under all PT Wd Plates which are on top of Concrete

1 1/2"

FIRST & SECOND FLOOR 100 PSF **SNOW LOADS:** GROUND SNOW LOAD 50 PSF MAX SNOW DRIFT LOAD 51 PSF WIND LOAD: 105 MPH BASIC WIND SPEED

SEISMIC DESIGN CATEGORY

RISK CATEGORY WIND EXPOSURE **EXPOSURE B** SEISMIC LOAD: SEISMIC IMPORTANCE FACTOR 1.0 SHORT PERIOD (Ss) 0.159 LONG PERIOD (S1) SITE CLASS

C3. ALL REINFORCEMENT SHALL BE SECURELY HELD IN PLACE FOR CONCRETE PLACEMENT. ADDITIONAL BARS OR STIRRUPS SHALL BE PROVIDED FOR SUPPORT OF ALL BARS AS REQUIRED.

C4. LAP CONTINUOUS REINFORCEMENT AS FOLLOWS, U.O.N. (ASSUMES CONCRETE COVER GREATER THAN TWO BAR DIAMETER, CENTER-TO-CENTER BAR SPACING GREATER THAN THREE BAR DIAMETERS):

4000 PSI CONCRETE:

TOP BARS* OTHER BARS 43"

*TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.

DC3. FOUNDATION CONDITIONS ASSUMED FOR FOUNDATION DESIGN INCLUDE AN ALLOWABLE SOIL BEARING CAPACITY OF 3 KSF; MAXIMUM HEIGHT OF WATER TABLE 4-FT BELOW FINISHED GRADE; AND FROST DEPTH OF 4 FT.

C5. TERMINATE ALL CONTINUOUS BARS WITH STANDARD HOOKS.

C6. MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: CAST AGAINST FORMS AND EXPOSED TO EARTH OR WEATHER: #5 AND SMALLER:

NOT EXPOSED TO WEATHER OR EARTH, SLABS, WALLS, AND JOISTS: #14 AND LARGER: 1 1/2" #11 AND SMALLER: 1 " **BEAMS AND COLUMNS:** 1 1/2"

C7. CONSTRUCTION JOINTS ARE NOT PERMITTED EXCEPT AS SHOWN ON THE DRAWINGS OR AS REVIEWED BY THE ENGINEER, SLAB

PLACEMENT SHALL NOT EXCEED 4000 SQ. FT. AND WALL PLACEMENT 80 FT. UNLESS APPROVED. C8. ALL CONCRETE SLABS-ON-GRADE SHALL HAVE MINIMUM WWF 6 X 6/W2.1 X W2.1 PER 4 INCH THICKNESS U.O.N.

C9. MINIMUM 8 MIL POLYETHYLENE VAPOR BARRIER SHALL BE INSTALLED UNDER ALL CONCRETE SLABS-ON-GRADE U.O. C10. CONTRACTOR TO LOCATE AND COORDINATE ALL INSERTS, SLOTS, SLEEVES, OPENINGS, PIPES, ETC. AS REQUIRED. C11. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE OR WHICH INCREASE THE POTENTIAL FOR CORROSION OF

MBEDDED METAL ITEMS SHALL NOT BE USED IN ANY CONCRETE.

C12. ALUMINUM ITEMS SHALL NOT BE PLACED IN CONCRETE. C13. PIPE OR CONDUIT EMBEDDED IN SLAB SHALL NOT EXCEED 1/3 THE SLAB THICKNESS AND SHALL BE PLACED WITHIN THE SLAB MIDDLE THIRD OF THICKNESS. MINIMUM CLEAR SPACING OF CONDUIT/PIPE IS 3 X OD. NO CONDUIT/PIPE TO BE PLACED CLOSER

THAN 12" FROM COLUMN FACE.
C14. CONCRETE DESIGN IS BASED ON ULTIMATE STRENGTH DESIGN OF ACI 318-14.

C15. WELDING OF REINFORCING BARS IS NOT PERMITTED EXCEPT BY PRIOR REVIEW OF THE ENGINEER.

C16. CONTRACTOR IS RESPONSIBLE FOR PROPER AND ADEQUATE SHORING OF CONCRETE WORK.

Project:

Salam Mosque Front Renovation PHASE 2

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Revisions:

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| | Rev. | Description | Ву | Date |
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Sheet Title:

Front Elevation

Project Manager: RH Project Architect: RH Drawn by: Checked by: Date Issued: 6-30-21



Scale: $\frac{1}{4}$ " = 1'-0"

Drawing No.: